



**Banha University**  
**Faculty of Engineering - Shoubra**  
**Civil Engineering Department**

**Computation of Nonlinear (STR602)**  
**For Master of Engineering Sciences**

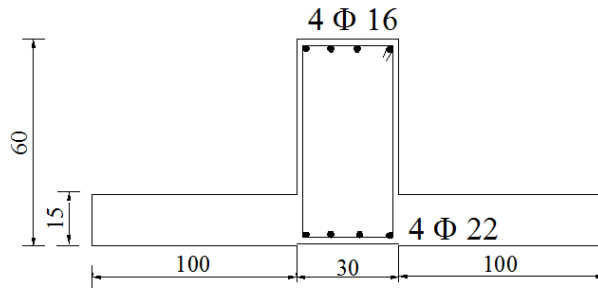
*Assoc. Prof. Taha Ibrahim*

*Lecture 4*

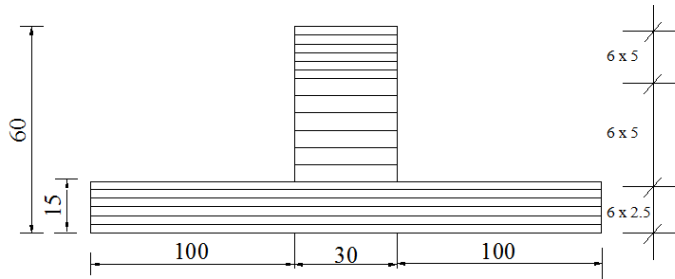


For the given section shown in Figure 1, the axial strain at mid height of the section  $\epsilon_0 = -0.0009$  and the slope  $\phi = -0.0001$  ( $d' = d'' = 2.5$  cm). Using the given stress-strain curves for steel and concrete in tension and compression, it is required to:

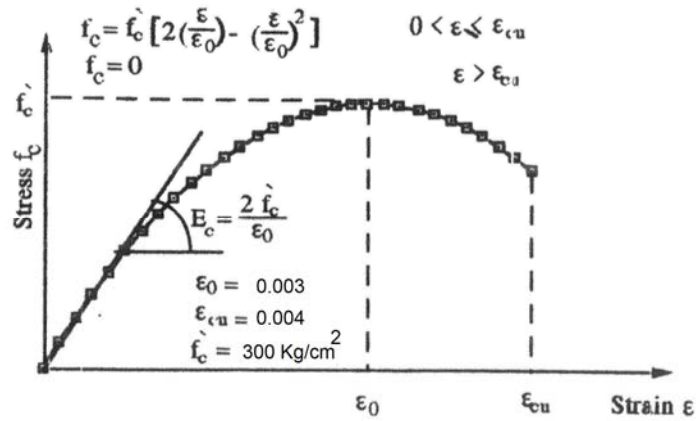
- Calculate and draw the strain distribution;
- Calculate and draw the stress distribution;
- Calculate axial, coupling and flexural stiffness's (A, B and D) using the secant modulus of elasticity;
- Calculate section capacity (M and N);



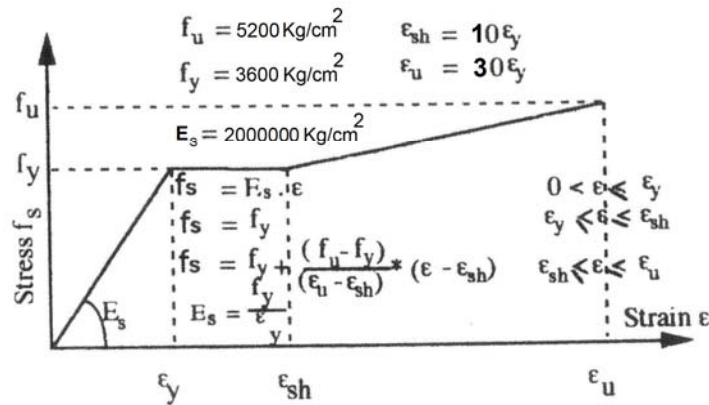
**Beam Section**  
 $(d' = d'' = 2.5 \text{ cm})$



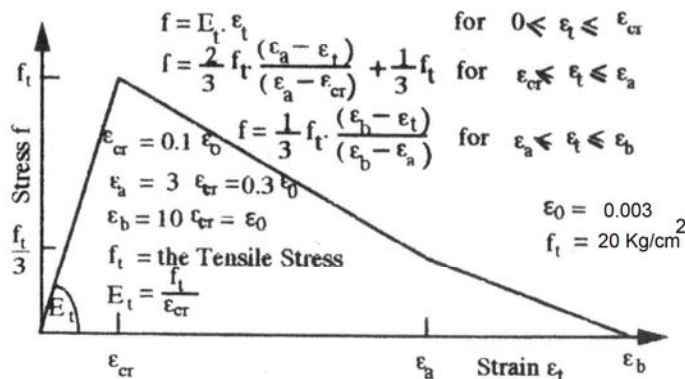
**Layered System**



**Concrete Stress-Strain Curve in Compression**



**Trilinear Stress-Strain Curve for Steel Reinforcement in Tension and Compression**



**Trilinear Model for Concrete in Tension**

## Axial stiffness (A) , Coupling stiffness (B) & Flexural stiffness (D)

comp. concrete given		
Fc' =	300	Kg/cm <sup>2</sup>
Fyst	2400	Kg/cm <sup>2</sup>
ε <sub>0</sub> =	0.003	
ε <sub>cu</sub> =	0.004	

Tension concrete given		
F <sub>cu</sub> =	300	Kg/cm <sup>2</sup>
F <sub>t</sub> =	20	Kg/cm <sup>3</sup>
ε <sub>0</sub> =	0.003	
ε <sub>cr</sub> =	0.0003	
E <sub>t</sub>	66666.667	Kg/cm <sup>2</sup>

Steel given		
St 37	360/520	
F <sub>y</sub>	3600	Kg/cm <sup>2</sup>
F <sub>u</sub>	5200	Kg/cm <sup>2</sup>
E <sub>s</sub>	2000000	Kg/cm <sup>2</sup>
ε <sub>y</sub> =	0.0018	
ε <sub>u</sub> =	0.054	
ε <sub>sh</sub> =	0.018	

ε <sub>cr</sub> =	0.0003
ε <sub>a</sub> =	0.0009
ε <sub>b</sub> =	0.003
E <sub>t</sub>	66666.66667

axial strain at mid height ε<sub>0</sub>= -0.0003

slope = -0.0001

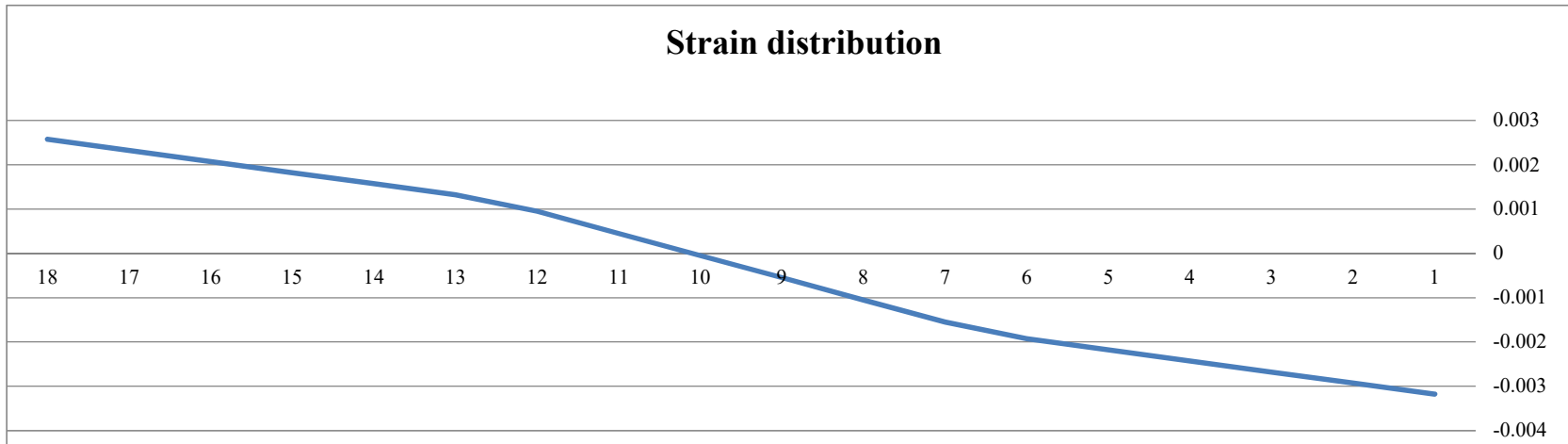
b=	230	cm	t=	60	cm	A <sub>s</sub> =	6.084	cm <sup>2</sup>
						A <sub>s</sub> '=	3.218	cm <sup>2</sup>

layer no.	layer type	T <sub>i</sub> (CM)	b <sub>i</sub> (CM)	z <sub>i</sub> (CM)	ε <sub>i</sub>	status	F <sub>i</sub> (Kg/CM <sup>2</sup> )	E <sub>secant</sub> (Kg/CM <sup>2</sup> )	A <sub>secant</sub> (cm <sup>2</sup> )	B <sub>secant</sub> (Kg.CM)	D <sub>secant</sub> (Kg.CM <sup>2</sup> )	N.F <sub>secant</sub> (Kg)	B.M <sub>secant</sub> (Kg.CM)
1	concrete	2.5	230	-28.75	-0.003175	c-comp	-298.9791667	94166.66667	54145833.33	-1556692708	44754915365	-171913.0208	4942499.349
2	steel	2.5	6.083	-26.25	-0.002925	steel	-3600	1230769.231	18716923.08	-491319230.8	12897129808	-54747	1437108.75
3	concrete	2.5	230	-23.75	-0.002675	c-comp	-296.4791667	110833.3333	63729166.67	-1513567708	35947233073	-170475.5208	4048793.62
4	concrete	2.5	230	-21.25	-0.002425	c-comp	-288.9791667	119166.6667	68520833.33	-1456067708	30941438802	-166163.0208	3530964.193
5	concrete	2.5	230	-18.75	-0.002175	c-comp	-277.3125	127500	73312500	-1374609375	25773925781	-159454.6875	2989775.391
6	concrete	2.5	230	-16.25	-0.001925	c-comp	-261.4791667	135833.3333	78104166.67	-1269192708	20624381510	-150350.5208	2443195.964
7	concrete	5	30	-12.5	-0.001550	c-comp	-229.9166667	148333.3333	22250000	-278125000	3476562500	-34487.5	431093.75
8	concrete	5	30	-7.5	-0.001050	c-comp	-173.25	165000	24750000	-185625000	1392187500	-25987.5	194906.25
9	concrete	5	30	-2.5	-0.000550	c-comp	-99.91666667	181666.6667	27250000	-68125000	170312500	-14987.5	37468.75
10	concrete	5	30	2.5	-0.000050	c-comp	-9.916666667	198333.3333	29750000	74375000	185937500	-1487.5	-3718.75
11	concrete	5	30	7.5	0.000450	c-ten	16.66666667	37037.03704	5555555.556	41666666.67	312500000	2500	18750
12	concrete	5	30	12.5	0.000950	c-ten	6.507936508	6850.459482	1027568.922	12844611.53	160557644.1	976.1904762	12202.38095
13	concrete	2.5	30	16.25	0.001325	c-ten	5.317460317	4013.177598	300988.3199	4891060.198	79479728.21	398.8095238	6480.654762
14	concrete	2.5	30	18.75	0.001575	c-ten	4.523809524	2872.260015	215419.5011	4039115.646	75733418.37	339.2857143	6361.607143
15	concrete	2.5	30	21.25	0.001825	c-ten	3.73015873	2043.922592	153294.1944	3257501.631	69221909.65	279.7619048	5944.940476
16	concrete	2.5	30	23.75	0.002075	c-ten	2.936507937	1415.184548	106138.8411	2520797.476	59868940.05	220.2380952	5230.654762
17	steel	2.5	3.217	26.25	0.002325	steel	3600	1548387.097	12452903.23	326888709.7	8580828629	28953	760016.25
18	concrete	2.5	30	28.75	0.002575	c-ten	1.349206349	523.9636308	39297.27231	1129796.579	32481651.64	101.1904762	2909.22619
<b>Σt= 60</b>									<b>ΣA=</b>	<b>ΣB=</b>	<b>ΣD=</b>	<b>ΣN.F=</b>	<b>ΣB.M=</b>
									480380588.9	-7721711180	1.85535E+11	-916285.2946	20869982.98

**C)      A= 480380588.9      B= -7721711180      D= 1.85535E+11**

**d)      N= -916285.2946      M= 20869982.98**

**a) Strain distribution**



**b) Stress distribution**

